Wedler Engineering LLP 201-9300 Nowell Street Chilliwack, BC V2P 4V7



May 5, 2020 File Ref: C19-5621/A

Fraser Valley Regional District 45950 Cheam Avenue Chilliwack, BC V2P 1N6

Attention: David Bennett, Planner

Reference: 52964 Yale Rd., Rosedale, B.C

**Drainage Design Brief** 

#### Summary

The following outlines the proposed storm water management plan for the commercial development proposed at 52964 Yale Road. The existing development south of the proposed development constructed a storm sewer system that provided service to the subject lot. This included storm sewer, storm water treatment systems, and infiltration facilities.

# 1. Existing Development

The existing development south of the proposed development was designed by MJL Engineering Ltd., Project No. 212012, dated October 12, 2012. The design was for a proposed gas station, roadway design, and future development for five different buildings. The design included water, sanitary, and storm services provided for each location, and labeled future if it was to be built at a later time. The proposed storm system at the time of development included storm sewers, catch basins, storm water treatment, and infiltration facilities.

#### 2. Water treatment

An oil/silt treatment facility is located on the development south of the project site and just before the infiltration facility. This treatment facility appears to have been designed to treat all of the run-off entering the infiltration facility.

# 3. Detention/Infiltration Design

The existing design from MJL Engineering's design proposed an infiltration rock pit. The rock pit was designed to include the proposed development, as well as other areas, at a runoff coefficient of 0.90. This information can be seen on the attached sheet SK01. Calculating the pervious and impervious area of the new commercial development, the weighted average run-off coefficient is as follows:

 Pervious area (R=0.30):
 369 sq. m

 Impervious area (R=0.90):
 3501 sq. m

 Total area:
 3870 sq. m

Weighted Run-off Coefficient: (369x0.30) + (3501x0.90)/3870 = 0.84

The required infiltration system storage volume that is required for the proposed development, including the catchment areas originally planned for the existing infiltration system, is 219 m<sup>3</sup>. Refer to the attached spreadsheets for calculations.

The infiltration storage volume provided by the existing infiltration system is 221 m<sup>3</sup>. Therefore, there is adequate capacity in the existing infiltration system to handle the storm water run-off for the proposed development.

## 4. Signature

We trust that this information meets your expectations. Should you have any questions or concerns, please contact the undersigned at your convenience.

Yours truly,

Wedler Engineering LLP

Per: Reviewed by:

André Gagné, P. Eng.
Patrick Mango Project Engineer

Civil Design Engineer <u>agagne@wedler.com</u>

cc: Steven Cross, X Architecture
Jay Lee, Rhee Ga Holdings

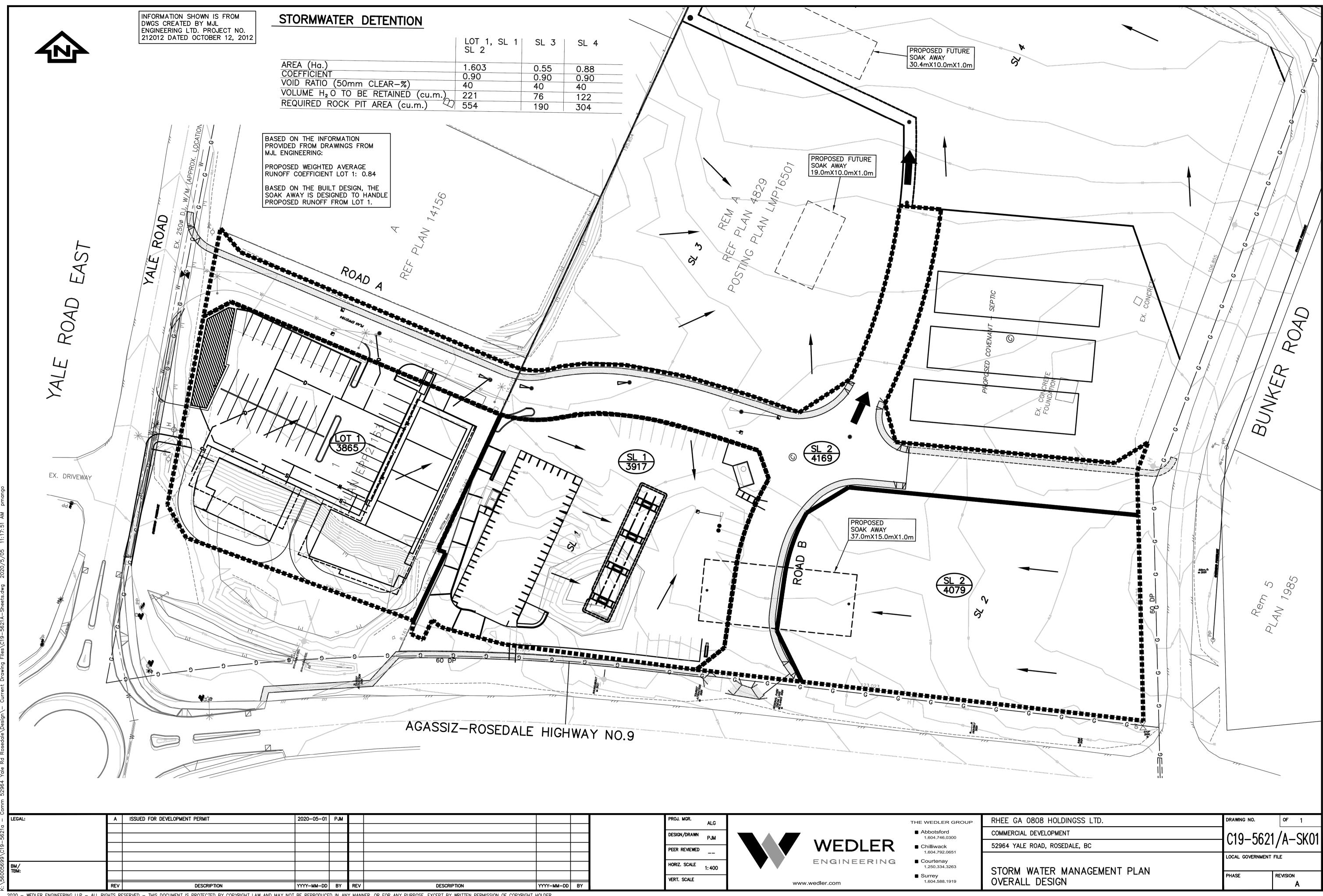
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#### **Enclosures:**

- Wedler Drawing No. C19-5621/A-SK01: Storm Water Management Plan Overall Design
- Wedler Engineering Soak-Away Sizing Calculation Sheet



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# SOAK-AWAY SIZING CALCULATION SHEET

Client: Rhee Ga Holdings Date Printed: May 5, 2020

Project: 52964 Yale Rd. Commercial Development By: PJM

Project #: C19-5621/A

Soak-Away Desc: Existing Soak Away - Updated Design

Area (m²) Runoff Coefficient

Infiltration Rate Used 144 mm/hr Length 37.00 m Lot 1 (Proposed) 3870 0.84 Width 15.00 m Ex. SL1 3917 0.9 Depth 1.00 m Fut. SL2 8248 0.9 **Footprint** 555 m<sup>2</sup> Combined 16035 0.89 Perf. Pipe Size 150 mm Perf Pipe Storage 0.7 m³

Storage Provided (40% void ratio) 222.4 m³
Max. Storage Req'd 219.6 m³

Discharge Data

Allowable Offsite Release 0.0000 m³/s None

Release rate from infiltration 0.0222 m³/s

Time	Intensity	Qin	Qout	Vol.in	Vol.out	Vin-Vout
min	mm/hr	m³/s	m³/s	m³	m³	m³
0	0.00	0.0000	0.0000	0.00	0.00	0.00
5	64.16	0.2533	0.0222	75.98	6.66	69.32
10	45.56	0.1798	0.0222	107.90	13.32	94.58
15	37.29	0.1472	0.0222	132.47	19.98	112.49
30	26.48	0.1045	0.0222	188.13	39.96	148.17
60	18.80	0.0742	0.0222	267.16	79.92	187.24
120	13.35	0.0527	0.0222	379.40	159.84	219.56
360	7.76	0.0306	0.0222	661.48	479.52	181.96
720	5.51	0.0217	0.0222	939.37	959.04	-19.67
1440	3.91	0.0154	0.0222	1334.01	1918.08	-584.07